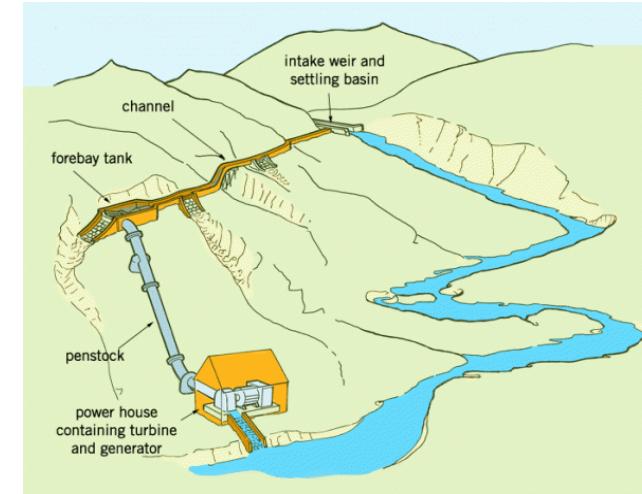


Dimensionamento preliminar do circuito hidráulico, energia anual média produzida e potência a instalar: conceitos básicos. Estimativa de custos

Maria Manuela
Portela
(2020/2021)

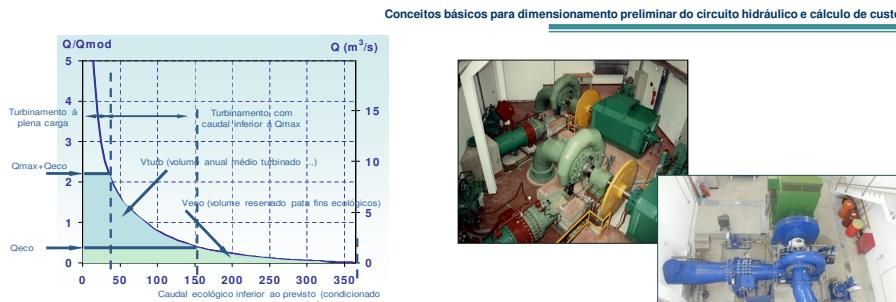


Cálculo da energia produzida e da potência a instalar

Dimensionamento das componentes principais do circuito hidráulico

Computation of the produced energy and of the installed capacity.

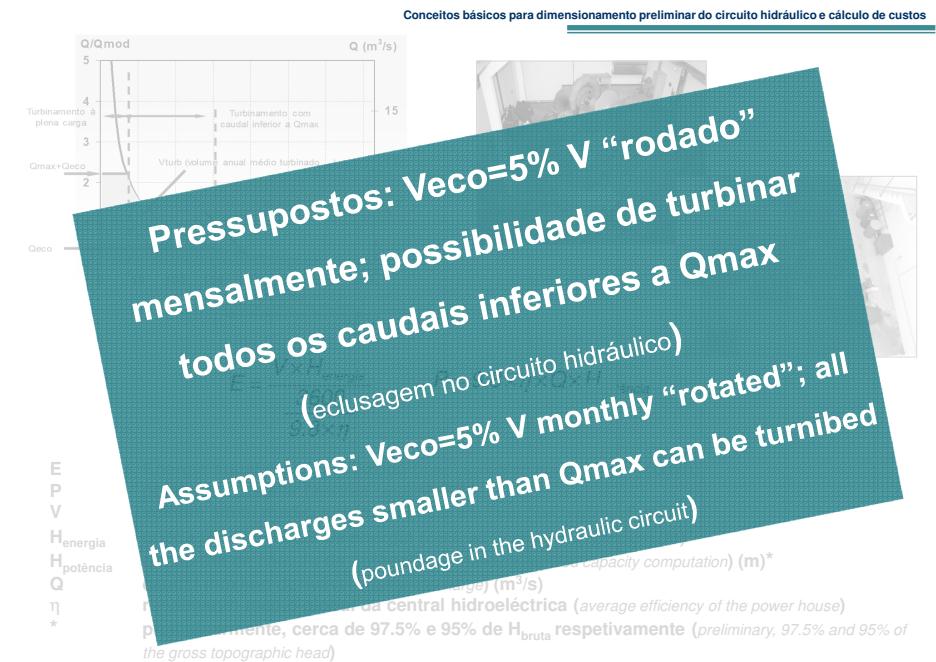
Design of the main components of the hydraulic circuit

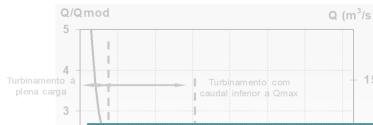


$$E = \frac{V \times H_{energia}}{3600}$$

$$P = 9.8 \times \eta \times Q \times H_{potência}$$

E	Energia anual média produzida (mean annual energy production) (GWh)
P	potência a instalar (installed capacity) (kW)
V	volume anual médio turbinado (mean annual turbinated volume) (hm ³)
H _{energia}	queda útil de cálculo da energia (net head for energy computation) (m)*
H _{potência}	queda útil de cálculo da potência (net head for installed capacity computation) (m)*
Q	caudal máximo derivável (design discharge) (m ³ /s)
η	rendimento médio global da central hidroeléctrica (average efficiency of the power house)
*	preliminarmente, cerca de 97.5% e 95% de H_{bruta} respetivamente (preliminary, 97.5% and 95% of the gross topographic head)





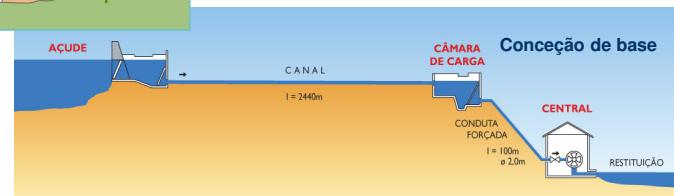
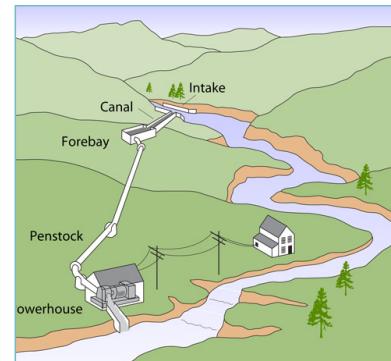
Queda topographic bruta → aprox. igual à diferença entre a cota do canal e a cota do rio na secção de restituição da central

(gross topographic head → approx. the elevation of the canal minus the elevation of the river at the tailrace of the power house)

queda útil de cálculo da potência (net head for installed capacity computation) (H)
caudal máximo derivável (design discharge) (m^3/s)
rendimento médio global da central hidroeléctrica (average efficiency of the power house)
preliminarmente, cerca de 97.5% e 95% de H_{bruta} respetivamente (preliminary, 97.5% and 95% of the gross topographic head)

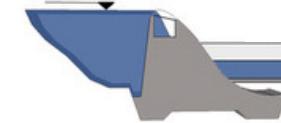


Dimensionamento das principais componentes do circuito hidráulico (design of the main components of the hydraulic circuit)



Açude munido de tomada de água do tipo tirolês

(weir with a Tyrolean water intake)



Açude munido de tomada de água do tipo tirolês

Função: (1) criar carga hidráulica que permita a admissão de caudais até Q_{max} , no circuito hidráulico; (2) permitir o posicionamento do trecho inicial do circuito hidráulico acima do nível da cheia de projeto no curso de água imediatamente a jusante do açude; (3) comportar órgãos anexos, incluindo tomada de água para o circuito hidráulico, descarga de fundo, dispositivo para lançamento do caudal ecológico e escada de peixes.

Function: (1) to create the hydraulic head that allows the admission of the different discharges, until Q_{max} , into the hydraulic circuit; (2) to prevent, under flood conditions, the overtopping of the initial reach of the canal immediately downstream the weir by placing it above the design flood level; (3) to allow the installation of auxiliary elements like gates, trash racks, ecological and fish pass ways



Nível de retenção normal, Nrn
h2

Alçado esquemático do açude

Somente na zona do talvegue/descarregador de superfície

Cota do leito do rio*
2.5 m
2.0 m (profundidade média da escavação)

Perfil transversal esquemático do rio na zona do açude
Largura do vale +/- (h1+h2) m acima do leito do rio, L2* + 4 m para encastramento lateral
Largura do leito do rio, L1*

* Larguras a deduzir com base na cartografia disponível e, no caso de L2, dependentes do declive das encostas

Nível de retenção normal, Nrn
h2

- ✓ Pressuposto $h1 \sim 11$ m (canal aprox. 10 m acima do leito do rio)
- ✓ $h2=(NMC-Nrn)+1 \rightarrow$ folga de 1 m entre o NMC e a cota do coroamento
- ✓ Encastramento lateral e na fundação de 2 m

Perfil transversal esquemático do rio na zona do açude
Largura do vale +/- (h1+h2) m acima do leito do rio, L2* + 4 m para encastramento lateral
Largura do leito do rio, L1*

* Larguras a deduzir com base na cartografia disponível e, no caso de L2, dependentes do declive das encostas

Normal retention level, Nrn
h2

Schematic cross section of the weir

Only downstream of the spillway
Cota do leito do rio*
2.5 m
2.0 m (average excavation depth)

Width of the valley +/- (h1+h2) m above the river bed, L2* + 4 m for lateral abutment anchorage
Width of the river bed, L1*

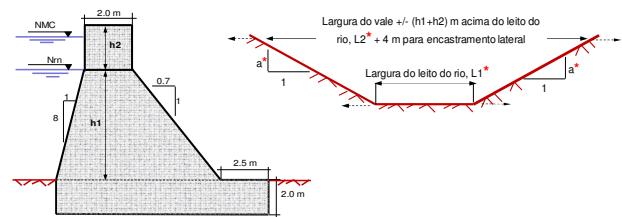
* Widths estimated based on the available topography and, regarding L2, also depending on the slopes of the lateral hillsides

Normal retention level, Nrn
h2

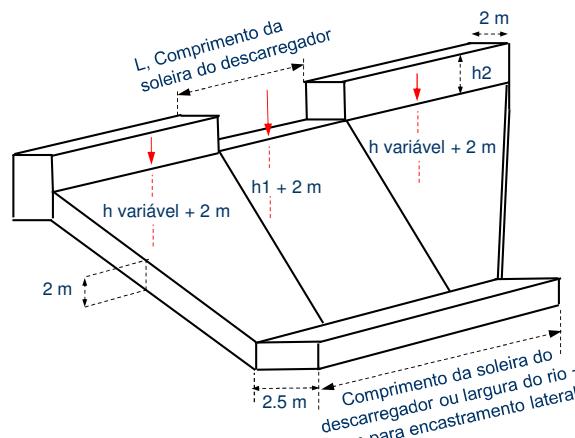
- ✓ Assumption $h1 \sim 11$ m (canal approx. 10 m above the river bed elevation at the section of the weir)
- ✓ $h2=(NMC-Nrn)+1 \rightarrow$ safety gap of 1 m between the NMC and the elevation of the crest of the weir
- ✓ Lateral and bottom "anchorage" of 2 m

Width of the valley +/- (h1+h2) m above the river bed, L2* + 4 m for lateral abutment anchorage
Width of the river bed, L1*

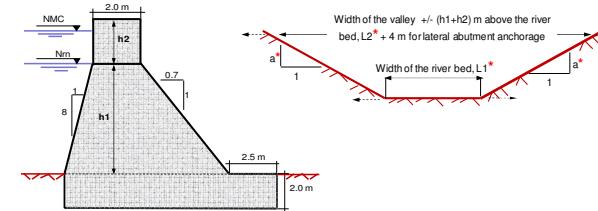
* Widths estimated based on the available topography and, regarding L2, also depending on the slopes of the lateral hillsides



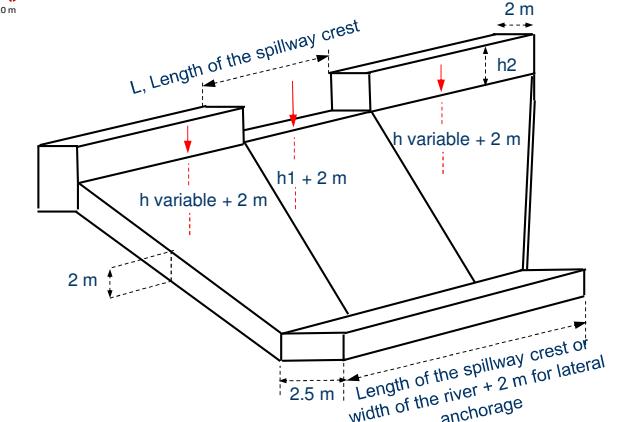
**Volume de betão
cálculo aproximado
tendo em conta a
volumetria 3D do
açude**



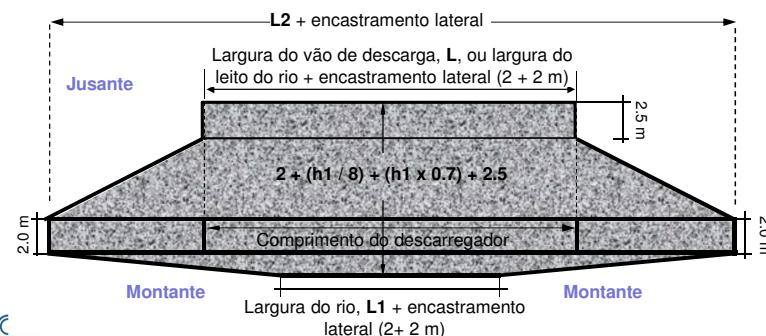
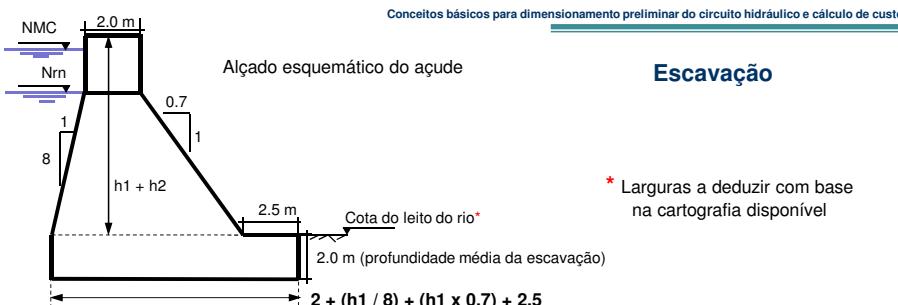
424



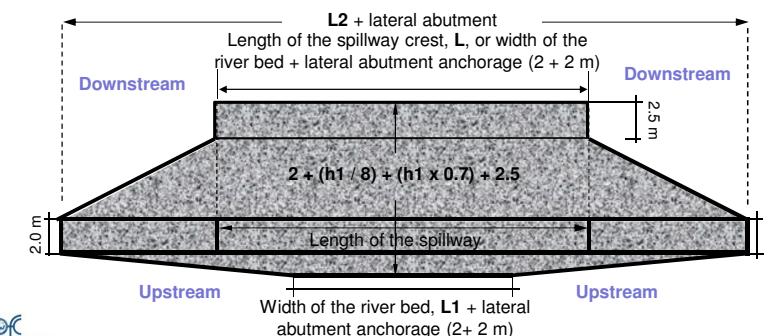
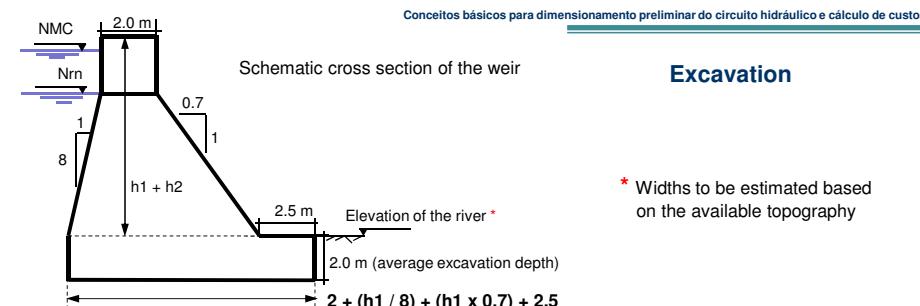
**Computation of the
concrete volume
taking into account
the 3D shape of the
weir**



425



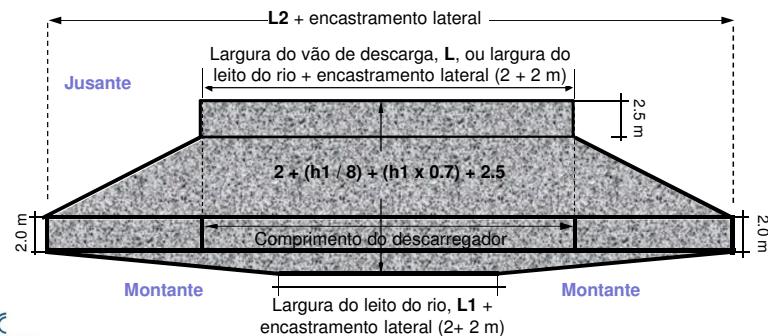
426



427

Escavação

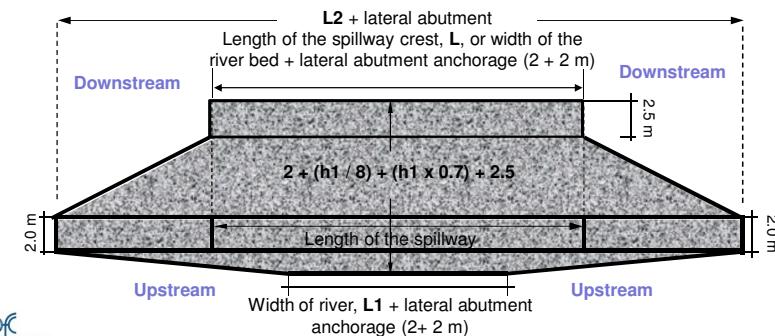
Volume de escavação ~ área em planta x profundidade da escavação (2m)



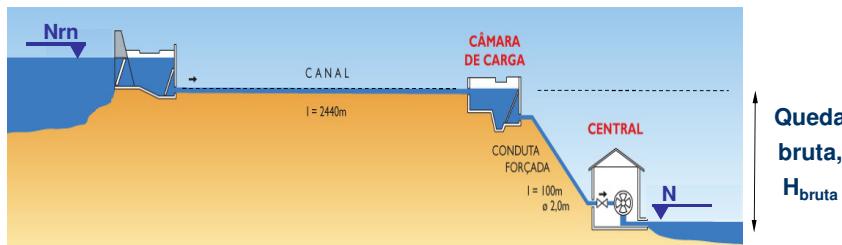
428

Excavation

Excavation volume ~ area of the horizontal projection x excavation depth (2m)



429



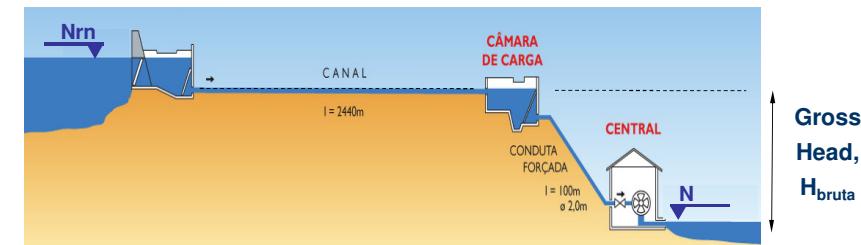
... preliminarmente ...

Nrn: Nível de retenção normal = cota do talvegue na secção de implantação do açude + h1 (~ 11 m)

Cota do coroamento do açude = cota dos muros de ala laterais ▼ (tem de conter uma folga de pelo menos 1 m relativamente ao nível da cheia centenária sobre a soleira do açude) = Nrn + Hc + 1 = NMC + 1



430



... preliminary ...

Nrn: Normal retention level = river bed elevation at the weir section + h1 (~ 11 m)

Elevation of the crest of the lateral abutments▼ (at least a safety gap of 1 m in relation to the water depth over the spillway of the 100-year flood) = Nrn + Hc + 1 = NMC + 1



431

Cálculo da altura de água sobre a soleira descarregadora em condições de cheia (descarga livre não controlada)

$$Qp = C L \sqrt{2 g} Hc^{3/2}$$

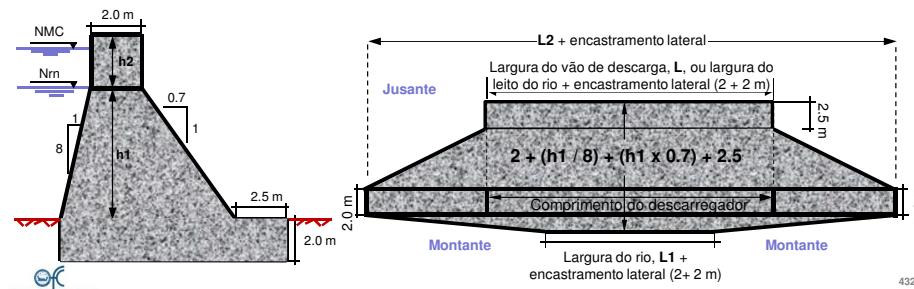
Qp caudal de ponta da cheia centenária (m^3/s) (2ª parte)

C coeficiente de vazão (0.48)

L comprimento da soleira descarregadora (m, L)

Hc carga sobre a crista da soleira descarregadora igual à diferença entre os níveis de máxima cheia, NMC, e o nível de retenção normal, Nrn (m)

$$Hc = NMC - Nrn \quad \longleftrightarrow \quad NMC = Nrn + Hc$$



432

Cálculo da altura de água sobre a soleira descarregadora em condições de cheia (descarga livre não controlada)

$$Qp = C L \sqrt{2 g} Hc^{3/2}$$

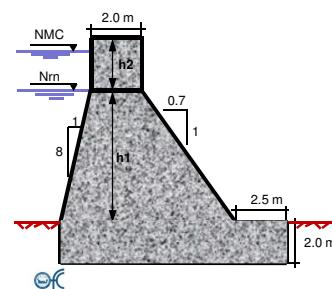
Qp caudal de ponta da cheia centenária (m^3/s) (2ª parte)

C coeficiente de vazão (0.48)

L comprimento da soleira descarregadora (m, L)

Hc carga sobre a crista da soleira descarregadora igual à diferença entre os níveis de máxima cheia, NMC, e o nível de retenção normal, Nrn (m)

$$Hc = NMC - Nrn \quad \longleftrightarrow \quad NMC = Nrn + Hc$$



434

Tendo em conta a pequena altura do açude (apenas $h1=11$ m), para evitar o seu eventual derrube Hc não deve exceder 3 a 3.5 m, se compatível com a largura do vale. Isto é, se o comprimento da soleira do descarregador não exceder a largura do vale, de modo a evitar a escavação excessiva das margens do rio

Cálculo da altura de água sobre a soleira descarregadora em condições de cheia (descarga livre não controlada)

$$Qp = C L \sqrt{2 g} Hc^{3/2}$$

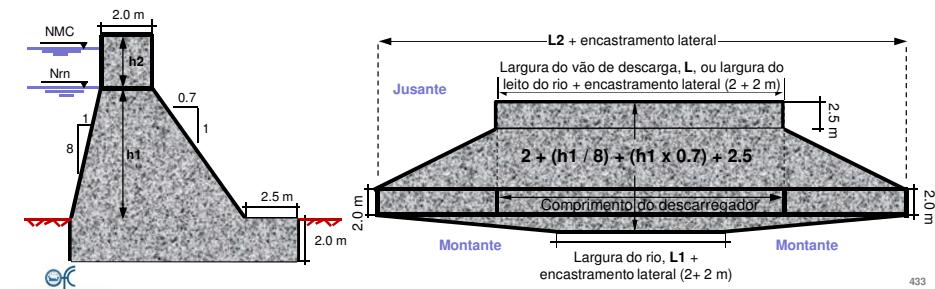
Qp caudal de ponta da cheia centenária (m^3/s) (2ª parte)

C coeficiente de vazão (0.48)

L comprimento da soleira descarregadora (m, L)

Hc carga sobre a crista da soleira descarregadora igual à diferença entre os níveis de máxima cheia, NMC, e o nível de retenção normal, Nrn (m)

$$Hc = NMC - Nrn \quad \longleftrightarrow \quad NMC = Nrn + Hc$$



433

Computation of the water depth over the spillway under flood conditions (uncontrolled spillway)

$$Qp = C L \sqrt{2 g} Hc^{3/2}$$

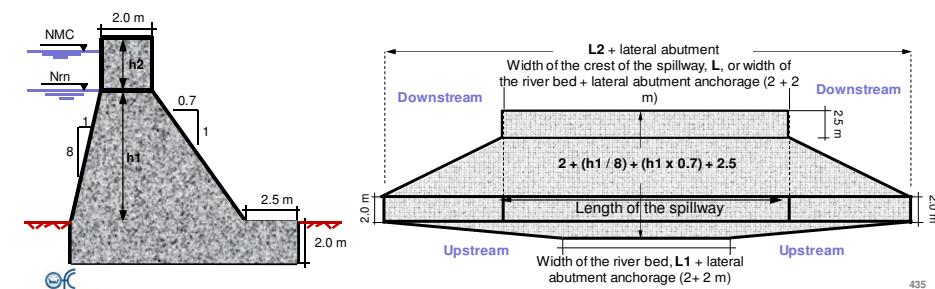
Qp 100-year peak flood discharge (m^3/s) (2nd Part)

C discharge coefficient (0.48)

L length of the crest of the spillway (m, L)

Hc head over the crest of the spillway equal to the difference between the maximum flood level, NMC, and the normal retention level, Nrn (m)

$$Hc = NMC - Nrn \quad \longleftrightarrow \quad NMC = Nrn + Hc$$



435

Computation of the water depth over the spillway under flood conditions (uncontrolled spillway)

$$Q_p = C L \sqrt{2 g} Hc^{3/2}$$

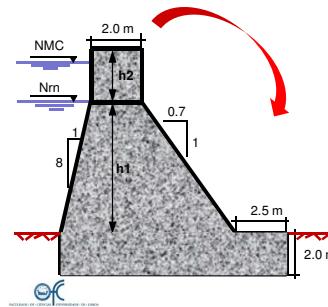
Q_p 100-year peak flood discharge (m^3/s) (2nd Part)

C discharge coefficient (0.48)

L length of the crest of the spillway (m, L)

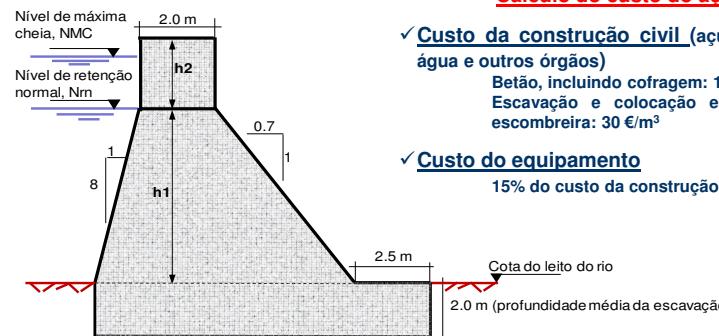
Hc head over the crest of the spillway equal to the difference between the maximum flood level, NMC, and the normal retention level, Nrn (m)

$$Hc = NMC - Nrn \quad \longleftrightarrow \quad NMC = Nrn + Hc$$



Due to the small height of the weir (only $h_1=11$ m), to prevent its overturning Hc must not exceed 3 to 3.5 m, if compatible to the width of the valley. I.e., if the length of the crest of the spillway do not exceed the width of the valley in order to avoid excessive the excavation of the river margins

436



Cálculo do custo do açude

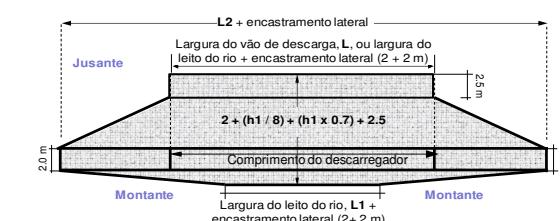
✓ Custo da construção civil (açude, tomada de água e outros órgãos)

Betão, incluindo cofragem: 120 €/m³

Escavação e colocação em depósito em escombreira: 30 €/m³

✓ Custo do equipamento

15% do custo da construção civil



437

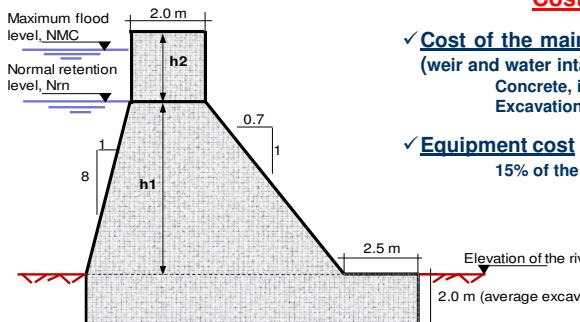
Cost of the weir

✓ Cost of the main civil construction works (weir and water intake)

Concrete, including formworks: 120 €/m³
Excavation and deposition: 30 €/m³

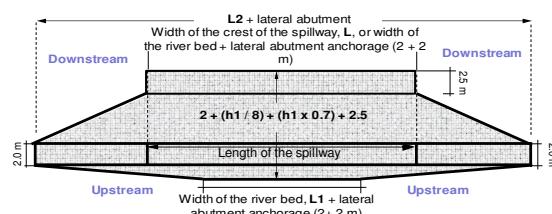
✓ Equipment cost

15% of the civil construction cost



Elevation of the river

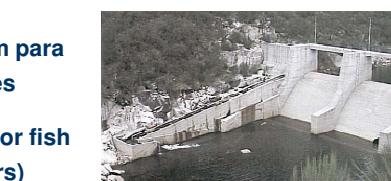
2.0 m (average excavation depth)



438



Passagem para peixes
(fishways or fish ladders)

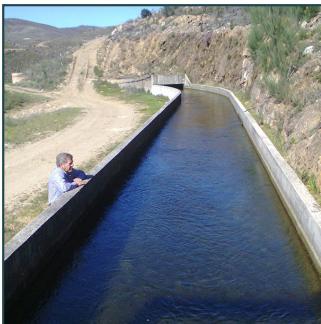


439



Conceitos básicos para dimensionamento preliminar do circuito hidráulico

Canal (adução em superfície livre)



Canal (free surface flow)

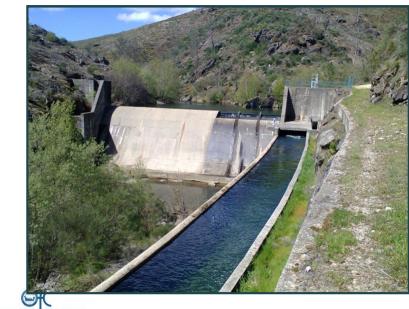
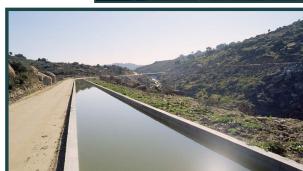
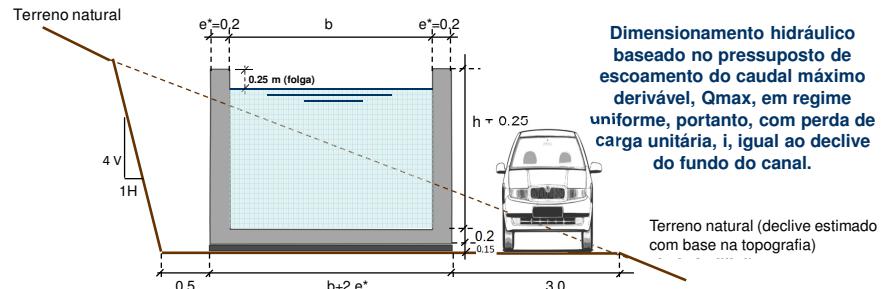


Foto: M. C. G. Lopes



440



✓ Pressupostos

- Declive do fundo do canal: 1 m/1 km ($i = 0.001 = 0.1\%$)
- Largura mínima do canal (rastro do canal): $b \geq 0.5\text{ m}$
- $b/h=1.5$, sendo h a altura do escoamento uniforme de Q_{\max} no canal
- Espessura das paredes do canal e da laje de fundo: $e^* = 0.20\text{ m}$
- Espessura do betão de regularização sob a laje de fundo: 0.15 m

✓ Fórmula de resistência ao escoamento – Fórmula de Manning-Strickler

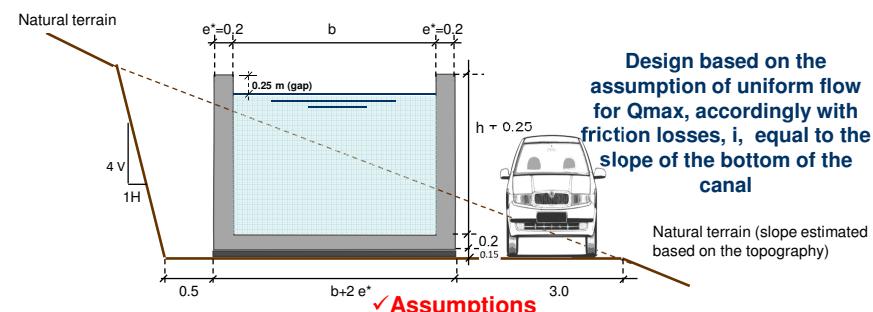
$$Q = K S R^{2/3} i^{1/2} \quad \text{com } R = \text{raio hidráulico} = S/P \quad S = \text{secção do escoamento}$$

$P = \text{perímetro molhado} \quad K = \text{coeficiente (para betão aprox. } 75 \text{ m}^{1/3} \text{ s}^{-1}\text{)}$

Etapas do dimensionamento



441



- Slope of the bottom of the canal: 1 m/1 km ($i = 0.001 = 0.1\%$)
- Minimum width of the canal: $b \geq 0.5\text{ m}$
- $b/h=1.5$, h being the water depth for Q_{\max} uniform flow
- Thickness of the walls of the canal (lateral and bottom): $e^* = 0.20\text{ m}$
- Thickness of the ground regularization concrete: 0.15 m

✓ Uniform flow head losses formula – Manning-Strickler formula

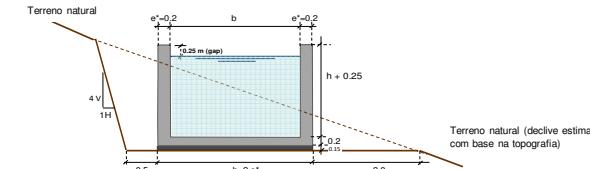
$$Q = K S R^{2/3} i^{1/2} \quad \text{with } R = \text{hydraulics radius} = S/P \quad S = \text{flow cross section}$$

$P = \text{wet perimeter} \quad K = \text{coefficient (75 m}^{1/3} \text{ s}^{-1} \text{ for concrete)}$

Design steps



442



✓ Pressupostos

- Declive do fundo do canal: 1 m/1 km ($i = 0.001 = 0.1\%$)
- Largura mínima do canal (rastro do canal): $b \geq 0.5\text{ m}$
- $b/h=1.5$, sendo h a altura do escoamento uniforme de Q_{\max} no canal
- Espessura das paredes do canal e da laje de fundo: $e^* = 0.20\text{ m}$
- Espessura do betão de regularização sob a laje de fundo: 0.15 m

✓ Fórmula de resistência ao escoamento – Fórmula de Manning-Strickler

$$Q_{\max} = K S R^{2/3} i^{1/2} \quad \text{com } R = \text{raio hidráulico} = S/P \quad K \text{ coeficiente (75 m}^{1/3} \text{ s}^{-1} \text{ para betão)}$$

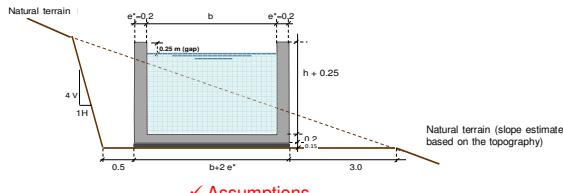
Etapas do dimensionamento

Arbítrio de um valor inicial para $b^{(1)}$ → $h^{(1)} = b^{(1)}/1.5$ → $S^{(1)} = b^{(1)} h^{(1)}$
 $P^{(1)} = 2 h^{(1)} + b^{(1)}$
 $R^{(1)} = S^{(1)}/P^{(1)}$

→ fórmula MS → $Q^{(1)} \leftrightarrow Q_{\max}$ → Se $Q^{(1)} > Q_{\max}$ → $b^{(2)} < b^{(1)}$
Se $Q^{(1)} < Q_{\max}$ → $b^{(2)} > b^{(1)}$

Repetição do procedimento até que $Q^{(i)} \sim Q_{\max}$ (Opção do Excel goal seek)

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✓ Assumptions

- Slope of the bottom of the canal: 1 m/1 km ($i = 0.001 = 0.1\%$)
- Minimum internal width of the canal: $b \geq 0.5$ m
- $b/h=1.5$, h being the water depth for Q_{max} uniform flow
- Thickness of the walls of the canal (lateral and bottom): $e^* = 0.20$ m
- Thickness of the ground regularization concrete: 0.15 m

✓ Uniform flow head losses formula – Manning-Strickler (MS) formula

$$Q_{max} = K S R^{2/3} i^{1/2} \quad \text{with } R = \text{hydraulics radius} = S/P \quad K \text{ coefficient (75 m}^{1/3} \text{s}^{-1} \text{ for concrete)}$$

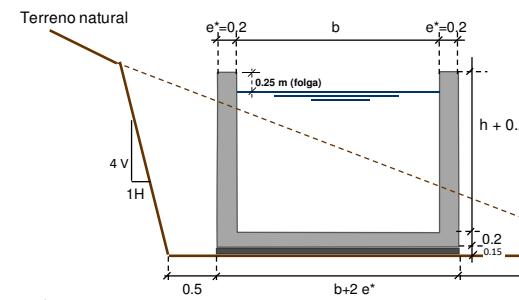
Design steps

Assumption of an initial value for $b^{(1)}$ $\rightarrow h^{(1)} = b^{(1)}/1.5 \rightarrow S^{(1)} = b^{(1)} h^{(1)}$
 $P^{(1)} = 2 h^{(1)} + b^{(1)}$
 $R^{(1)} = S^{(1)} / P^{(1)}$

→ MS formula $\rightarrow Q^{(1)} \leftrightarrow Q_{max}$ → If $Q^{(1)} > Q_{max}$ $\rightarrow b^{(2)} < b^{(1)}$
If $Q^{(1)} < Q_{max}$ $\rightarrow b^{(2)} > b^{(1)}$

Repetition of the procedure until $Q^{(i)} \sim Q_{max}$ (Excel goal seek option)

444



Custo do canal

✓ Pressupostos

- Declive do fundo do canal: 1 m/1 km ($i=0.001$)
- Largura mínima do canal (rasto do canal): $b \geq 0.5$ m
- $b/h=1.5$, sendo h a altura do escoamento uniforme de Q_{max} no canal
- Espessura das paredes do canal e da laje de fundo: $e=0.20$ m
- Espessura do betão de regularização sob a laje de fundo: 0.15 m

✓ Fórmula de resistência ao escoamento – Fórmula de Manning-Strickler

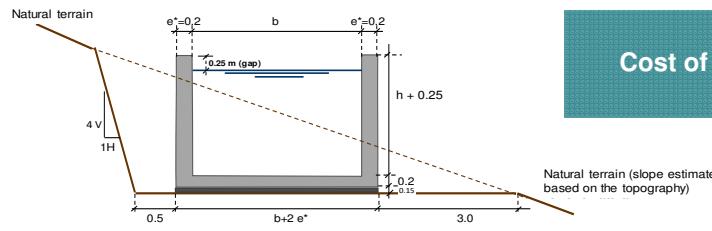
$$Q_{max} = K S R^{2/3} i^{1/2} \quad \text{com } R=\text{raio hidráulico}= S/P \quad K \text{ coeficiente (para betão aprox. } 75 \text{ m}^{1/3} \text{s}^{-1})$$

✓ Custo da construção civil

- Betão de regularização: 80 €/m³
- Betão armado: 200 €/m³
- Escavação e colocação em depósito: 30 €/m³
- Cofragem externa: 20 €/m² $\rightarrow 2(h + 0.25 + 0.20) \text{ m}^2/\text{m}$ (área por metro linear de canal)
- Cofragem interna: 20 €/m² $\rightarrow [2(h + 0.25)] \text{ m}^2/\text{m}$ (área por metro linear de canal)

✓ Custo do equipamento

- 5% do custo da construção civil



Cost of the canal

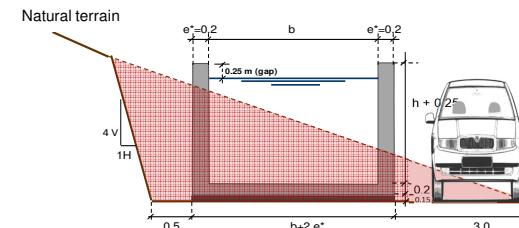
✓ Assumptions

- Slope of the bottom of the canal: 1 m/1 km ($i = 0.001 = 0.1\%$)
- Minimum internal width of the canal: $b \geq 0.5$ m
- $b/h=1.5$, h being the water depth for Q_{max} uniform flow
- Thickness of the walls of the canal (lateral and bottom): $e^* = 0.20$ m
- Thickness of the ground regularization concrete: 0.15 m

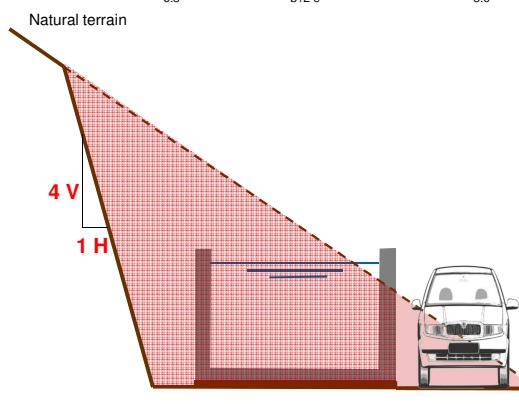
✓ Uniform flow head losses formula – Manning-Strickler formula

$$Q = K S R^{2/3} i^{1/2} \quad \text{with } R = \text{hydraulics radius} = S/P \quad S = \text{flow cross section}$$

$$P = \text{wet perimeter} \quad K = \text{coefficient (75 m}^{1/3} \text{s}^{-1} \text{ for concrete)}$$



Natural terrain



The excavation volume depends on the average slope of the hillsides of the natural terrain

AVERAGE SLOPE ALONG THE CANAL

446

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Rampas de salvamento do canal (canal rescue ramps)

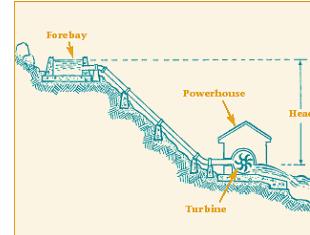


448

ONC
NACIONAL DE CÁLCULOS ECONÔMICOS DE CÁLCULOS

Câmara de carga (promove a transição entre o escoamento em superfície livre no canal e o escoamento em pressão na conduta forçada) – custo da construção civil aprox. igual ao custo de 100 m de canal; custo do equipamento igual a 25% do custo da construção civil

Forebay (it promotes the transition between the free surface flow in the canal and the pressurized flow in the penstock) – civil construction cost approx. equal to the cost of 100 m of canal; equipment cost equal to 25% of the civil construction cost



Upstream to downstream view

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Conceitos básicos para dimensionamento preliminar do circuito hidráulico e cálculo de custos

Conduta forçada em aço (adução em pressão) – instalada ao ar livre apoiada em maciços de apoio e dispondo de maciços de amarração nas singularidades em planta e em perfil

Steel penstock (pressurized flow) – installed in open air on support blocks and with anchor blocks in any profile singularity (curves either in plant or profile)



450



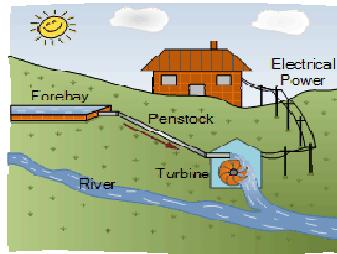
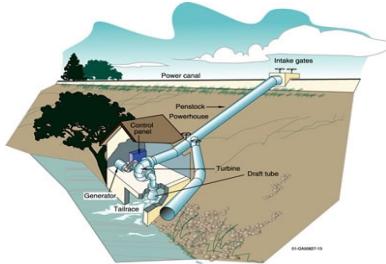
Conceitos básicos para dimensionamento preliminar do circuito hidráulico e cálculo de custos

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Steel penstock (pressurized flow) – installed in open air on support blocks and with anchor blocks in any profile singularity (curves either in plant or profile)



451



✓ Pressupostos

- dimensionamento resultante de otimização técnico econômica tendo em conta mas ...
- velocidade máxima do escoamento da ordem de 3 m/s - $Q = v S$
- diâmetros, D, comerciais disponíveis de 50 em 50 mm

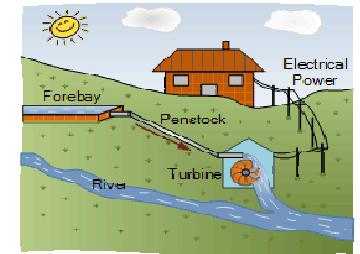
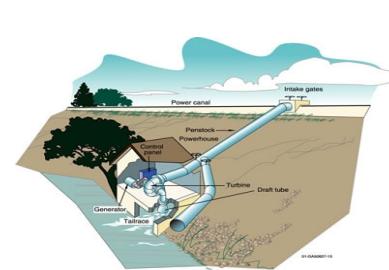
✓ Fórmula de resistência ao escoamento – Fórmula de Manning-Strickler

$$Q = K S R^{2/3} J^{1/2} \quad Q \text{ caudal (m}^3/\text{s)}; D \text{ diâmetro (m)}; S = \pi (D/2)^2 (\text{m}^2); R = D/4 \text{ raio hidráulico (m)}; J \text{ perda de carga unitária (-)}; K \text{ coeficiente (para aço aprox. 90 m}^{1/3} \text{ s}^{-1}\text{); } \Delta h = J L \text{ perda de carga (m)}$$

(Nota: analisar a perda de cota do rastro do canal adicionada com a perda de carga na conduta forçada para o caudal máximo derivável, $J L$, de modo a não exceder aprox. 5% da queda bruta. Relativamente a este limitar, o valor obtido deve de ter em conta ainda uma folga para atender às demais perdas de carga não contabilizadas, incluindo a perda de carga na aduão)

✓ Espessura da conduta - 1 m ca (coluna da água) = 0.1 kgf/cm²; $e_{corrosão} = 0.1 \text{ cm}$

$$e (\text{cm}) = \frac{1.5 \times H_{bruta} (\text{m})}{2400 (\text{kgf/cm}^2)} \times \frac{10}{D (\text{m})} \times 100 + e_{corrosão} (\text{cm})$$



✓ Assumptions

- design resulting from a technical economical optimization based on the cost of the energy losses along the same ... but ...
- maximum flow velocity of approx. 3 m/s - $Q = v S$
- commercial diameters, D, available each 50 mm (theoretical diameter rounded to the next higher commercial diameter)

✓ Flow resistance formula – Manning-Strickler formula

$$Q = K S R^{2/3} J^{1/2} \quad Q \text{ discharge (m}^3/\text{s)}; D \text{ diameter (m)}; S = \pi (D/2)^2 (\text{m}^2); R = D/4 \text{ hydraulic radius (m)}; J \text{ friction slope (-)}; K \text{ coefficient (for steel approx. 90 m}^{1/3} \text{ s}^{-1}\text{); } \Delta h = J L \text{ head loss (m)}$$

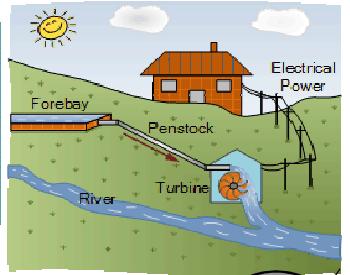
(Note: The sum of the decrease of the elevation of the bottom of the canal with the head loss along the penstock for the design discharge, $J L$, and with a safety gap of a couple of meters to account for all the remaining head losses along the hydraulic circuit should not exceed approx. 5% of the gross topographic head)

✓ Thickness of the penstock - 1 m wc (water column) = 0.1 kgf/cm²; $e_{corrosão} = 0.1 \text{ cm}$

$$e (\text{cm}) = \frac{1.5 \times H_{gross} (\text{m})}{2400 (\text{kgf/cm}^2)} \times \frac{10}{D (\text{m})} \times 100 + e_{corrosion} (\text{cm})$$



Custo da conduta baseado no custo do aço



✓ Peso da conduta

$$\text{peso (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times 2\pi \times [D(\text{m})/2 + e(\text{m})/2] \times e(\text{m})$$

$$\text{peso (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times \pi \times [D(\text{m}) + e(\text{m})] \times e(\text{m})$$

$$\gamma_a = 7800 \text{ kgf/m}^3 \quad (\text{peso específico do aço})$$



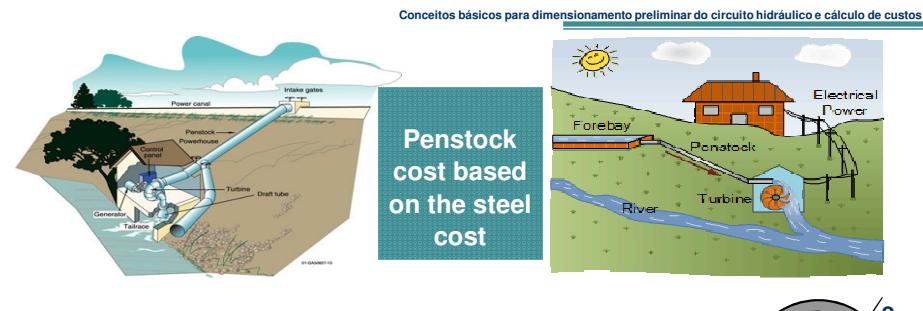
✓ Custo da conduta (equipamento) colocada em obra com base no custo do aço

$$- Cu_{aço} = 6 \text{ €/kgf}$$

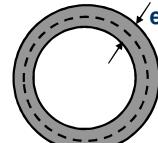
✓ Custo da construção civil por metro linear (maciços de apoio e de amarração)

$$- cu = 300 \text{ €/m}$$

Etapas do dimensionamento



Penstock cost based on the steel cost



✓ Weight of the penstock

$$\text{weight (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times 2\pi \times [D(\text{m})/2 + e(\text{m})/2] \times e(\text{m})$$

$$\text{weight (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times \pi \times [D(\text{m}) + e(\text{m})] \times e(\text{m})$$

$$\gamma_a = 7800 \text{ kgf/m}^3 \quad (\text{specific weight of the steel})$$

✓ Cost of the penstock (equipment) installed in situ based on the steel cost

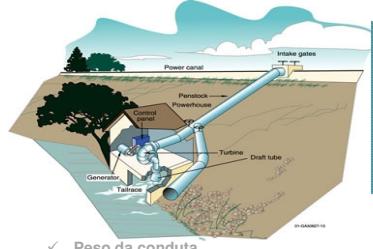
$$- Cu_{st} = 6 \text{ €/kgf}$$

✓ Cost of the civil construction by linear meter (support and anchor blocks)

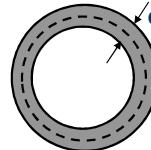
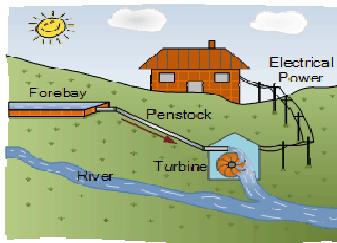
$$- cu = 300 \text{ €/m}$$

Design steps





Dimensio-namento da conduta



✓ Peso da conduta

$$\text{peso (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times 2\pi \times [D(m)/2 + e(m)/2] \times e(m)$$

$$\text{peso (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times \pi \times [D(m) + e(m)] \times e(m)$$

$$\gamma_a = 7800 \text{ kgf/m}^3 \text{ (peso específico do aço)}$$

✓ Custo da conduta (equipamento) colocada em obra com base no custo do aço

$$- Cu_{aço} = 6 \text{ €/kgf}$$

✓ Custo da construção civil por metro linear (maciços de apoio e de amarração)

$$- cu = 300 \text{ €/m}$$

Etapas do dimensionamento

$$Q_{max} + v = 3 \text{ m/s} \rightarrow Q_{max} = v S^* \rightarrow S^* = \pi (D_{teórico}/2)^2 \rightarrow D_{teórico} \rightarrow D_{comercial} \rightarrow S = \pi (D_{comercial}/2)^2 \rightarrow R_{hidráulico} = D/4$$

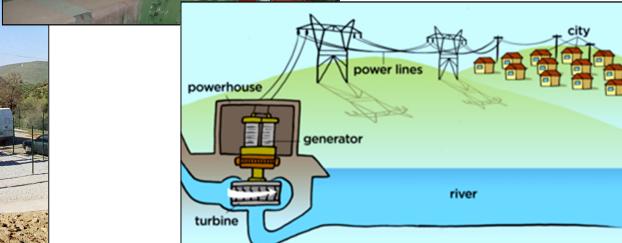
$$\text{formula de MS} \rightarrow J \text{ (perda de carga unitária)} \rightarrow \Delta h = J \times \text{comprimento real} = \text{Perda de carga na conduta forçada}$$

Compatível com o pressuposto de máxima perda de carga total de (1-0.95) da queda bruta (... atender à diminuição da cota do canal + folga para outras perdas de carga ...)

Não → próximo $D_{comercial}$

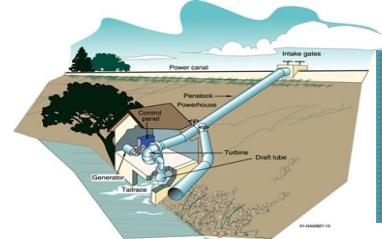
Sim → $D_{comercial}$

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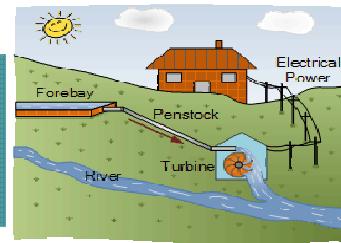


Central hidroelétrica

Powerhouse



Design of the penstock



✓ Weight of the penstock

$$\text{weight (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times 2\pi \times [D(m)/2 + e(m)/2] \times e(m)$$

$$\text{weight (kgf/m)} = \gamma_a (\text{kgf/m}^3) \times \pi \times [D(m) + e(m)] \times e(m)$$

$$\gamma_a = 7800 \text{ kgf/m}^3 \text{ (specific weight of the steel)}$$

✓ Cost of the penstock (equipment) installed in situ based on the steel cost

$$- Cu_{aço} = 6 \text{ €/kgf}$$

✓ Cost of the civil construction by linear meter (support and anchor blocks)

$$- cu = 300 \text{ €/m}$$

Design steps

$$Q_{max} + v = 3 \text{ m/s} \rightarrow Q_{max} = v S^* \rightarrow S^* = \pi (D_{theoretical}/2)^2 \rightarrow D_{theoretical} \rightarrow D_{commercial} \rightarrow S = \pi (D_{commercial}/2)^2 \rightarrow R_{hydraulic} = D/4$$

$$\text{MS formula} \rightarrow J \text{ (friction losses)} \rightarrow \Delta h = J \times \text{real length} = \text{penstock head losses}$$

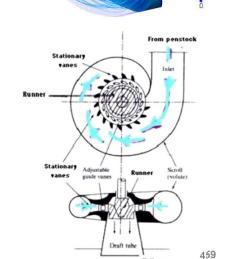
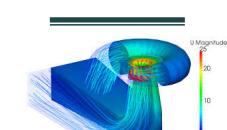
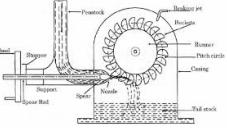
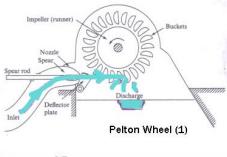
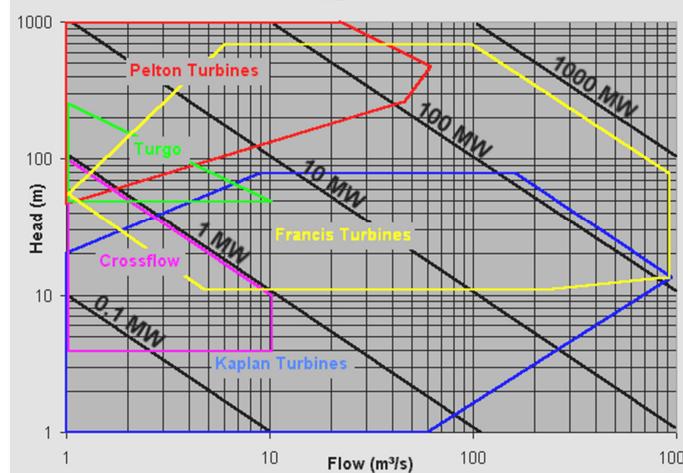
Compatible with the assumption of total maximum head losses of (1-0.95) of the gross head (... account for the decrease of the elevation of the canal bottom + a safety gap for additional head losses ...)

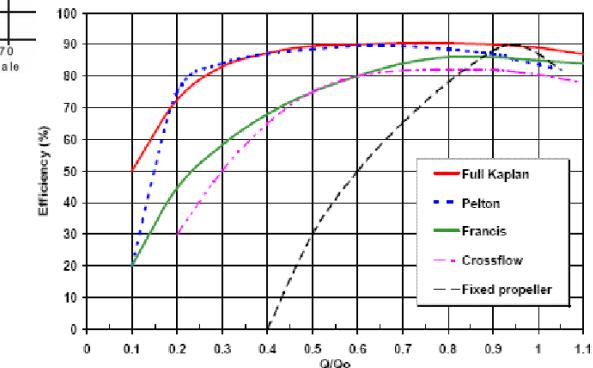
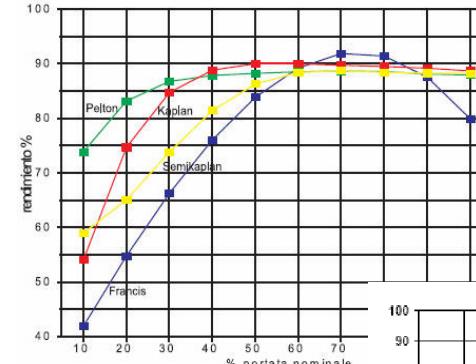
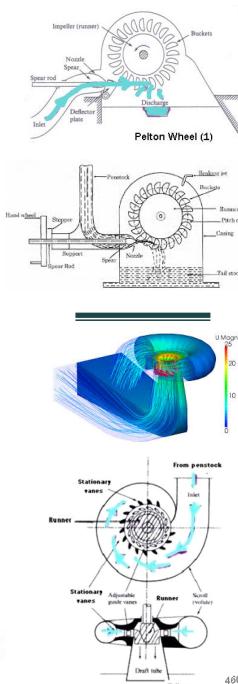
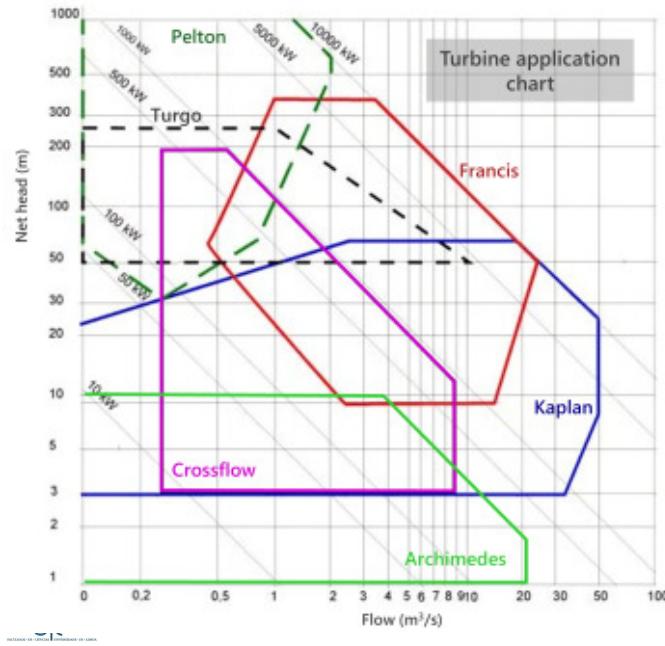
No → next $D_{commercial}$
Yes → $D_{commercial}$

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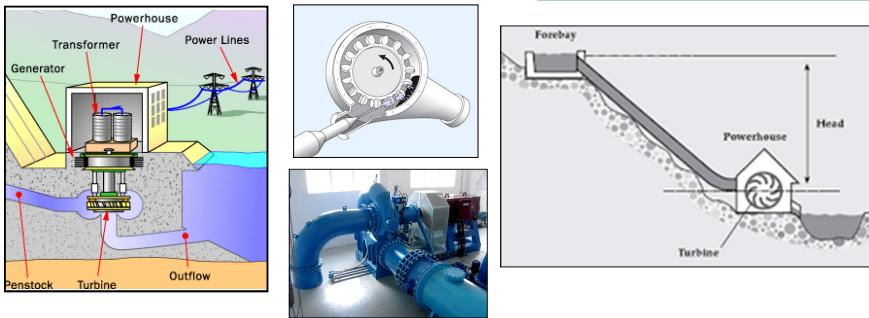


Turbine Application Chart





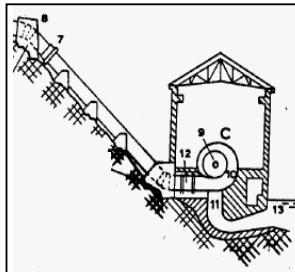
(fornecidas pelo fabricante)
(provided by the manufacturer)



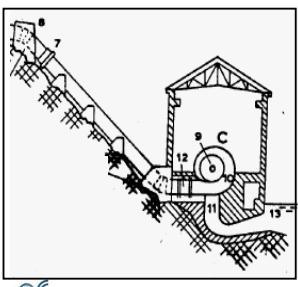
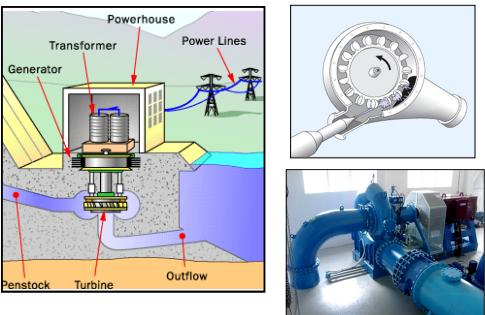
✓ Custo de uma central hidroeléctrica a céu aberto, incluindo subestação anexa:

$$ct = K_p P^{0.7} H_{\text{potência}}^{-0.35}$$

em que ct (€) é o custo total; K_p um fator igual a 5000000 ou a 4500000, para turbinas do tipo Pelton e do tipo Francis, respetivamente; P (MW) a potência instalada; e $H_{\text{potência}}$ (m) a queda de cálculo da potência. Do anterior custo total, admite-se que 75% sejam custos dos diferentes equipamentos instalados na central e 25%, custos com a construção civil.



turbinas do tipo Pelton e do tipo Francis, respetivamente; P (MW) a potência instalada; e $H_{\text{potência}}$ (m) a queda de cálculo da potência (m). Do anterior custo total, admite-se que 75% sejam custos dos diferentes equipamentos instalados na central e 25%, custos com a construção civil.

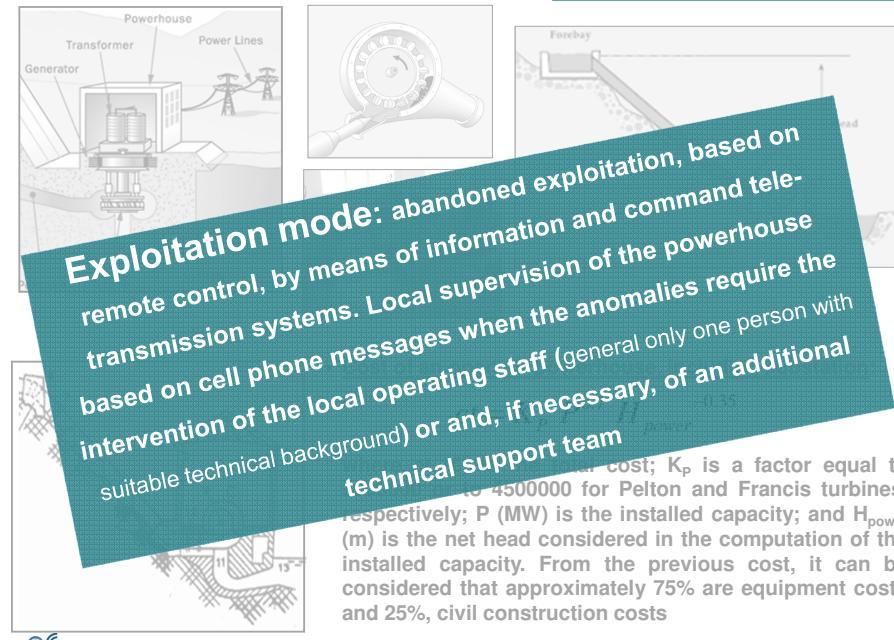


✓ Cost of an open air powerhouse, including substation:

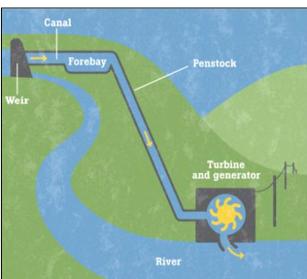
$$ct = K_p P^{0.7} H_{potência}^{-0.35}$$

where ct (€) is the total cost; K_p is a factor equal to 5000000 or to 4500000 for Pelton and Francis turbines, respectively; P (MW) is the installed capacity; and $H_{potência}$ (m) is the net head considered in the computation of the installed capacity. From the previous cost, it can be considered that approximately 75% are equipment costs and 25%, civil construction costs

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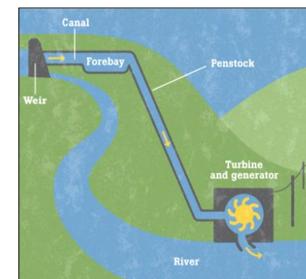
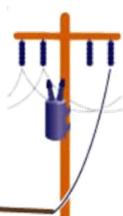


✓ Outros custos de investimento

- Estudos e projetos
- Fiscalização
- Ligação à rede elétrica nacional
- Acessos (obra e exploração)
- Imprevistos, estaleiro, ensecadeiras, desmatação/desflorestação, desvio provisório
- Aquisição de terrenos

✓ Custos anuais

- operação (~1.5 x salário mínimo)
- manutenção
- administrativos (2500 €/MW)
- de utilização e licenciamento (1% da receita anual média)



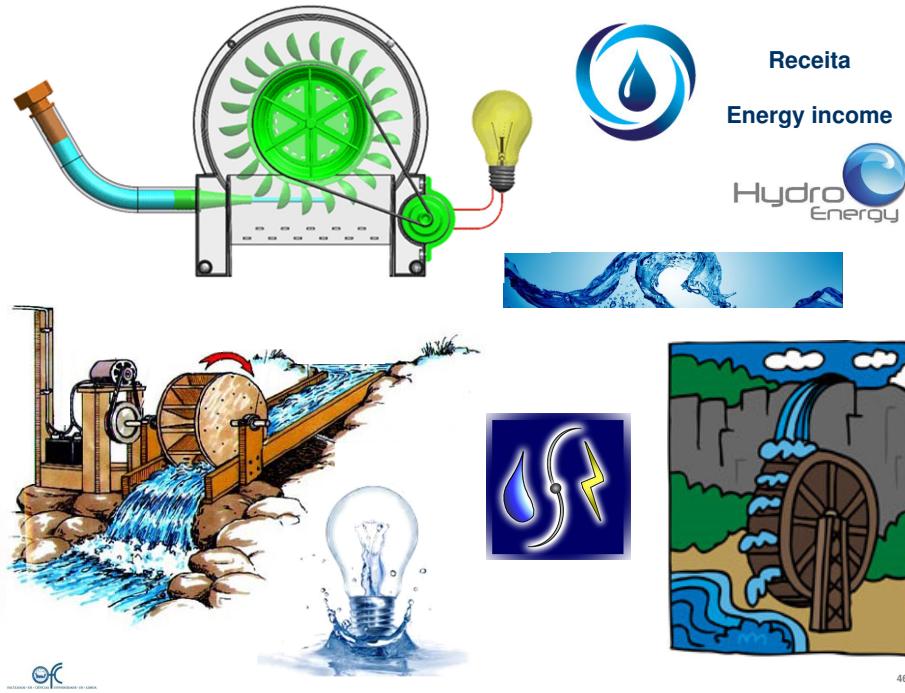
✓ Other investment costs

- Studies and designs
- Site supervision
- Connection to the electrical grid
- Roads (for construction and exploitation)
- Unforeseen, site facilities, coffer dams, river bypass, deforestation
- Land acquisition

✓ Annual costs

- Operation (~1.5 x minimum wage)
- Maintenance
- Administrative (2500 €/MW)
- Licensing and exploitation (1% the mean annual income)





CRITÉRIOS DE ANÁLISE ECONÓMICA
ANÁLISE DA VIABILIDADE ECONÓMICA DE SOLUÇÕES
ALTERNATIVAS DE UM MESMO PROJETO

Economical analysis criteria
Economical feasibility study of the alternative solutions
of a same project

Maria Manuela
Portela

Um projeto visa a **satisfação** de um dado objectivo representando, normalmente, uma das vias possíveis para satisfazer esse objectivo, vias por vezes concorrentes entre si ou até mutuamente exclusivas



A project aims at **satisfying a specific objective**, often representing one of the possible ways to comply with that same objective. Those ways often compete each other or even are mutually exclusive

Evaluation to which extend the project fulfills or not the objective

Definition of alternative solutions of a same project
all of them technically feasible

cost benefit value business

Comparison of the different solutions in a common basis

Economic analysis criteria

FLUXOS MONETÁRIOS (custos e receitas)

... numa dupla perspetiva: montante e ocorrência ao longo do tempo (calendarização). Quanto mais rigorosas forem as estimativas dos fluxos monetários inerentes ao projeto, tanto mais os resultados da análise económica poderão contribuir para a comparação de soluções e para a fundamentação de decisões ou seja, para a identificação, de entre projetos alternativos tecnicamente viáveis, do melhor projeto.



MONETARY FLOWS OR CASH FLOWS (cost and incomes)

... in a double perspective: **amount** and **occurrence in time** (time schedule). The more precise are the estimates of the monetary fluxes the more the results from the economic analysis will contribute to the comparison and to the identification of the more correct and profitable solution among the alternatives solution of a same project, all of them technically sound.

CONSTANT MARKET PRICE SYSTEM referred to a particularly year, generally the first year of the exploitation period – this price system avoids the consideration of the inflation by considering that it equally affects all the monetary components of the system.

Monetary flows: **investment cost**, including reposition cost, as costs occurring at specific moments; **exploitation and maintenance costs**, as annual cost; **incomes**,).

SISTEMA DE PREÇOS DE MERCADO CONSTANTES referidos a um dado ano, geralmente, o ano de início de exploração – tal sistema evita, em certa medida, a consideração da inflação admitindo que a mesma afeta de igual modo todas as componentes do projeto.

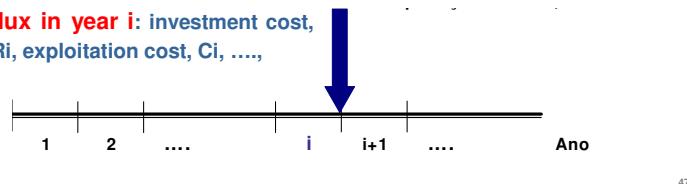
Custos de investimento, correspondendo, muito frequentemente, a gastos pontuais no tempo; custos de exploração e de manutenção, como encargos normalmente anuais; custos de reposição dos equipamentos e receitas

❖ PERÍODO DE ANÁLISE: período de tempo a que se refere a análise económica e para o qual são comparados (em termos económicos) os custos e as receitas tendo em vista apreciar a viabilidade do projeto ou identificar o projeto mais vantajoso (período de vida útil do projeto, horizonte de planeamento, questões relacionadas com o licenciamento...).

Ao longo deste período considera-se que os fluxos monetários são sempre atribuídos ao fim do ano a que se referem.

Period of analysis (often the lifetime of the most relevant components): period of time considered in the comparison - in economical terms - of the costs and of the benefits. The appraisal of the economic feasibility of the project and the comparison among alternative solutions of a same project is done for that period. **Any monetary flow during the period of analysis is always assigned to the end of the year to which it refers**

Monetary flux in year i: investment cost, ii, income, Ri, exploitation cost, Ci,,



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DISCOUNT RATE - THE INTEREST RATE USED IN THE DISCOUNTED CASH FLOW ANALYSIS TO DETERMINE THE PRESENT VALUE OF FUTURE AND PAST CASH FLOWS



Total Monthly Income	\$	
Savings	Monthly Amounts	
Savings Accounts	\$	
Individual Investments	\$	
Other	\$	
Other	\$	
Other	\$	
Total Savings	\$	-
Debt Payments		
Mortgage	\$	
Mortgage	\$	
Bank Loans	\$	
Car lease/Loan	\$	
Credit Cards	\$	
Other	\$	
Other	\$	
Total Debt Payments	\$	-
Fixed Expenses		
Home		
Real Estate Taxes	\$	
Maintenance Fees	\$	
Other	\$	

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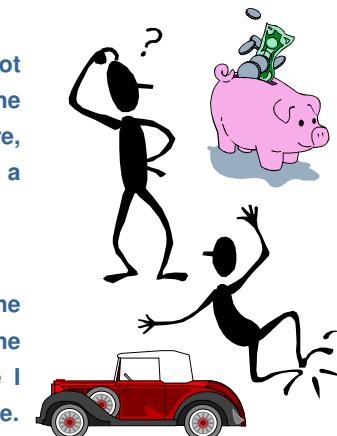
Taxa de atualização

Os fluxos monetários ocorrem em diferentes instantes: fluxos passados, presentes e futuros que não são diretamente comparáveis entre si. A transferência no tempo da possibilidade de dispor de um bem tem a característica de uma troca entre bens diferentes.

A taxa dessa troca desempenha o papel de uma relação de preço – taxa de atualização anual, t, prevalecente num certo momento (quanto estou disposto a deixar de consumir hoje para consumir no futuro ou quanto não consumirei no futuro por preferir consumir hoje).

DISCOUNT RATE

Cash flows occurring at different instants are not directly comparable. The transference of the ownership of a good from the present to the future, or vice-versa, has the characteristic of a asset/good exchange.



The rate of that exchange is the discount rate.

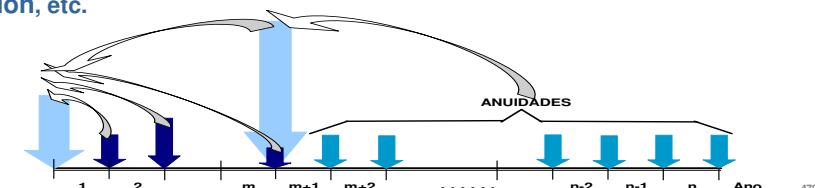
It measures how much I am willing to save in the present in order to spend in the future or, on the contrary, if I prefer to consume today because I know that it is not worthwhile to save for the future.

A taxa de atualização, t, permite calcular o valor que se atribui num dado instante a um fluxo monetário ocorrente num instante diferente.

A fixação da taxa de atualização, t, reflete a escolha entre consumir hoje ou no futuro e tem em conta, entre outros fatores, a taxa de juro do mercado, a disponibilidade de capitais, o risco associado ao projeto, a inflação esperada, etc.

The discount rate, t, allows to assign a present value to monetary flows/cash flows (referred to a market price system) that occurred in the past or will occur in the future.

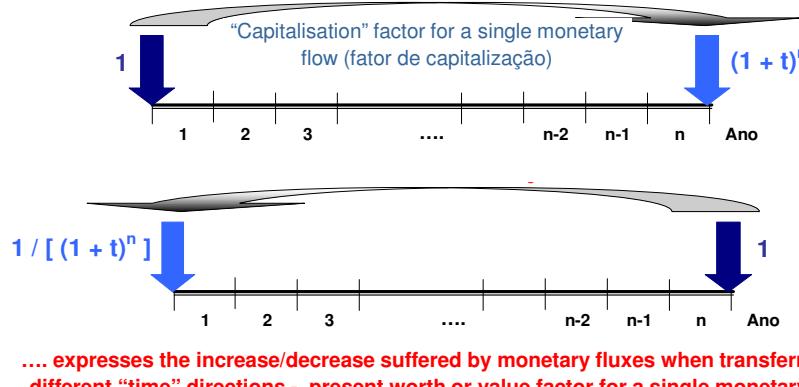
The value adopted for the discount rate, t, reflects the choice between consuming today instead of consuming in the future or vice-versa and takes into account, among other factors, the market interest rate, the availability of the capital, the bank interest rate, the risk of the project, the expected inflation, etc.



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Time transference of monetary flows/cash flows in a system of constant market prices

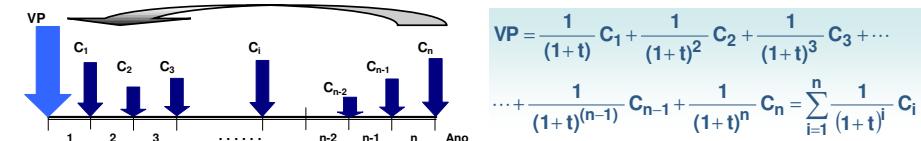
Let n denote the length (expressed in years) of the period of analysis, from year 1 to year n . One monetary unit of today will be change in year n by $(1+t)^n$ monetary units and one monetary unit of year n will be change today by $1/(1+t)^n$ units (discount factor)



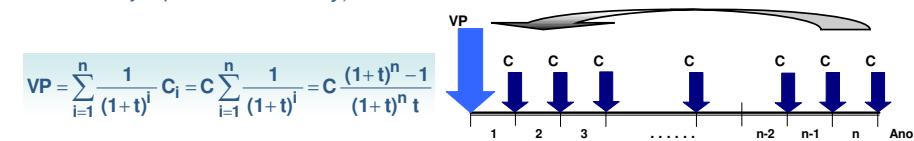
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1. Fator de atualização fornece a depreciação sofrida pelos fluxos monetários quando transferidos para o presente. O valor presente, VP, de uma unidade monetária que venha a ocorrer no ano i é dado por: $VP = \frac{1}{(1+t)^i} C_i$

2.O valor presente, VP, de uma sequência de custos C_i designa-se por valor acumulado atualizado para o início do 1º ano sendo dado por:



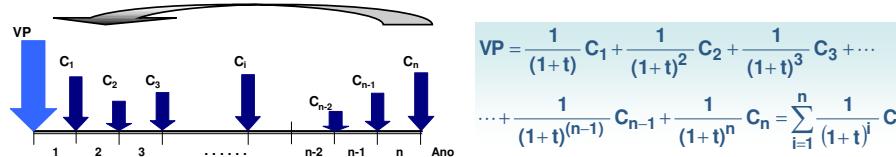
3.Ou, se os fluxos monetários forem constantes (or if the monetary fluxes are constant i.e., if they represent an annuity);



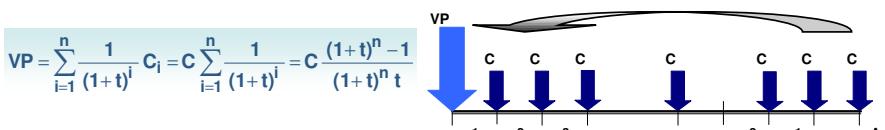
1.The “actualization” or discount factor evaluates the depreciation suffered by the monetary flows/cash flows when transferred from the future to the present. The present value, VP, of C_i monetary units assigned to the end of year i is given by:

$$VP = \frac{1}{(1+t)^i} C_i$$

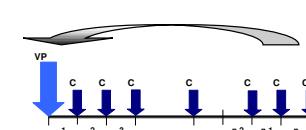
2.The present value of a sequence of monetary flows referred to the first year of the analysis period - accumulated net present value or simply net present value - is given by:



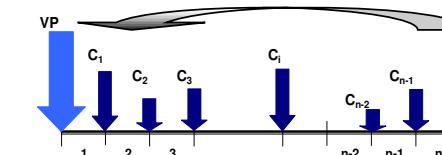
3.If the monetary fluxes are constant, i.e., if they represent an annuity:



VP: valor presente ou valor acumulado atualizado para o início do 1º ano

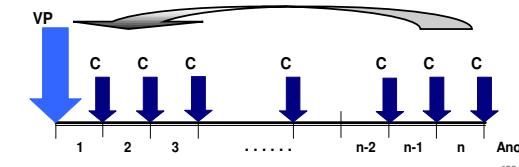


Fator de atualização à taxa t para o começo do período de n anos de uma série uniforme de fluxos monetários (anuidades) ocorrentes no fim de cada ano.



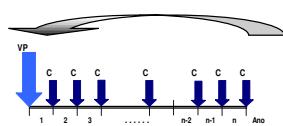
Custos ocorrentes no futuro (restituição de equipamentos).

Anuidades ocorrentes no futuro (receita, se avaliada na base da receita anual média).



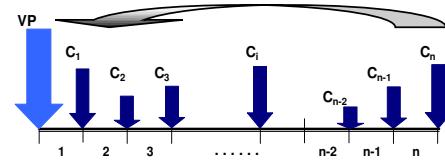
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Present value or cumulative present value, VP, referred to the beginning of the first year



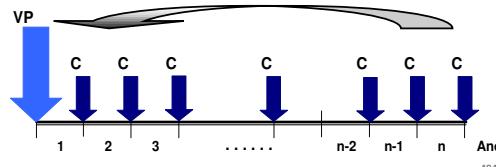
$$\frac{(1+t)^n - 1}{(1+t)^n t}$$

"Actualization" or **discount factor** for the discount rate t and for the beginning of the first year of the period with annuities, each one occurring at the end of each year: **present worth factor for an uniform series**



Future cost, e.g., reposition cost or O&M costs

Future annuities, e.g., expected annual incomes

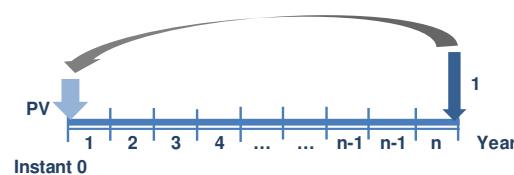


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SUMMARY

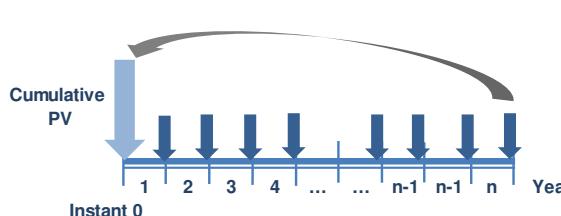
PRESENT VALUE, PV, OF FUTURE MONETARY FLOWS/CASH FLOWS

(all the monetary flows are referred to a constant market price system for the beginning of year 1 – instant 0 – and allocated to the end of the year in which they occur; the instant 0 is the end of year -1/beginning of year 1)



"Actualization" or discount factor for **single monetary flow in year n**

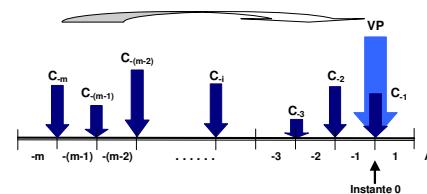
$$\frac{1}{(1+t)^n}$$



"Actualization" or discount factor for a **series of future annuities** during n years

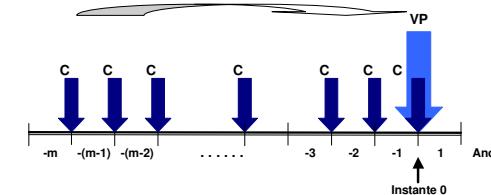
$$\frac{(1+t)^n - 1}{(1+t)^n t}$$

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$$VP = C_{-1} + C_{-2} (1+t) + C_{-3} (1+t)^2 + \dots + C_{-(m-1)} (1+t)^{(m-2)} + C_{-m} (1+t)^{(m-1)} = \sum_{i=1}^m (1+t)^{(i-1)} C_{-i}$$

$$VP = \sum_{i=1}^m (1+t)^{i-1} C_{-i} = C \sum_{i=1}^m (1+t)^{i-1} = C \frac{(1+t)^{(m-1)}}{t}$$



Fator de capitalização à taxa t para o termo do período de m anos de uma série uniforme de fluxos monetários (anuidades) ocorrentes no fim de cada ano

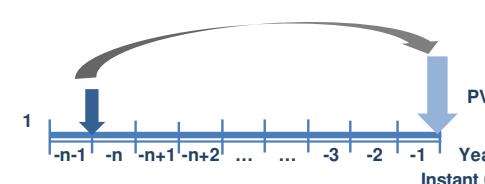
Capitalization factor for the discount rate t and for the end of the period of m years with annuities, each one occurring at the end of each year

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SUMMARY

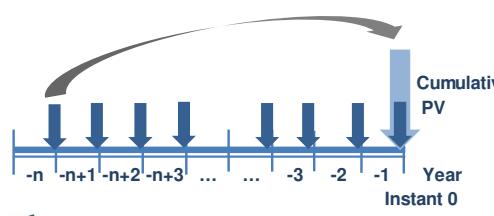
PRESENT VALUE, PV, OF PAST MONETARY FLOWS

(all the monetary flows are referred to a constant market prices for the beginning of year 1 – instant 0 – and allocated to the end of the year in which they occur; the instant 0 is the end of year -1/beginning of year 1)



"Actualization" or discount factor for **single monetary flow in year n**

$$(1+t)^n$$



"Capitalization" or discount factor for a **series of past annuities** during n years

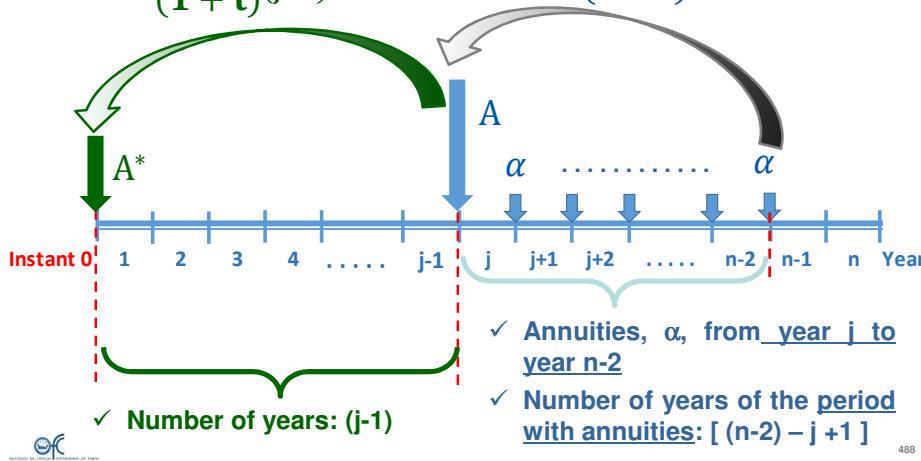
$$\frac{(1+t)^{(n-1)}}{t}$$

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Appointment

Present value referred to instant 0, A^* , of the sequence of annuities

$$A^* = A \frac{1}{(1+t)^{(j-1)}}$$



Cumulative present value, A , of the sequence of annuities – referred to the beginning of the first year with annuities

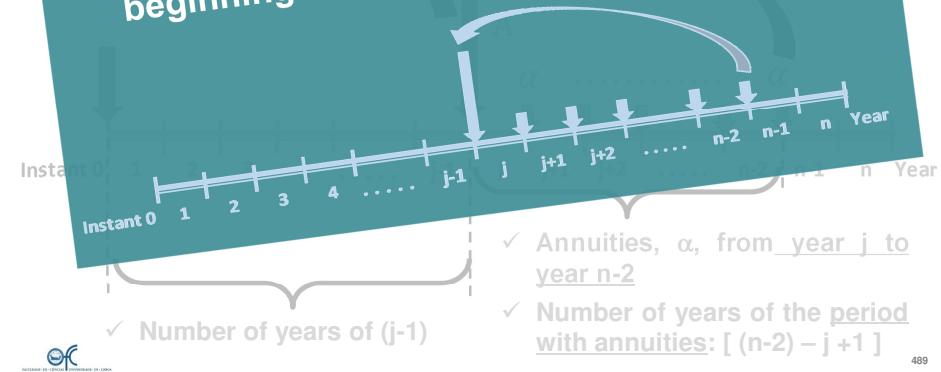
$$A = \alpha \frac{(1+t)^{[(n-2)-j+1]} - 1}{(1+t)^{[(n-2)-j+1]} t}$$

Appointment

Present value referred to instant 0, A^* , of the sequence of annuities

Cumulative present value, A , of the sequence of annuities – referred to the beginning of the first year with annuities

The discount factor always provides the present value of a sequence of units referred to the beginning of the first year with annuities



ECONOMIC INDICATORS

The assessment of the economic feasibility of a project technically sound or the comparison between alternative solutions of a same project all of them technically sound are based on economic indicators:

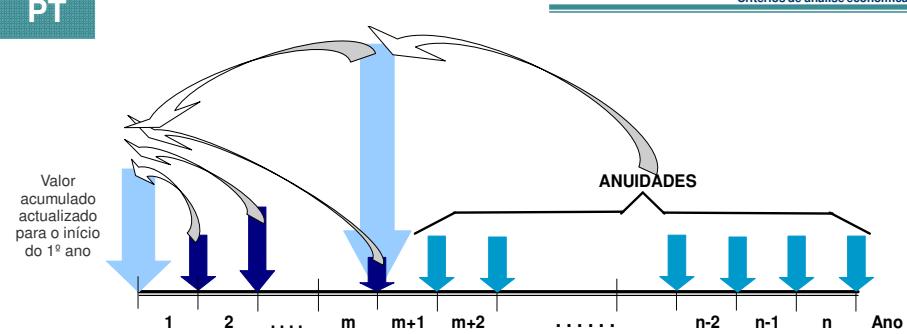
- Net present value, NPC (valor atualizado líquido, VAL);
- Benefit/cost ratio, B/C (índice benefício/custo);
- Internal rate of return, IRT (taxa interna de rentabilidade, TIR);
- Payback period (período de recuperação do investimento).

NOTATION:

- n lifetime of the project = period of analysis
- t discount rate (taxa de atualização)
- I_i Investment cost in year i
- O_i operation and maintenance cost, O&M, in year i;
- R_i income in year i
- S_i reposição cost in year i

ASSUMPTION: CONSTANT MARKET PRICE SYSTEM (referred to moment 0)

PT



$$I = \sum_{i=1}^m \frac{1}{(1+t)^i} I_i$$

Investimentos

$$O = \sum_{i=1}^{n-m} \frac{1}{(1+t)^i} O_i$$

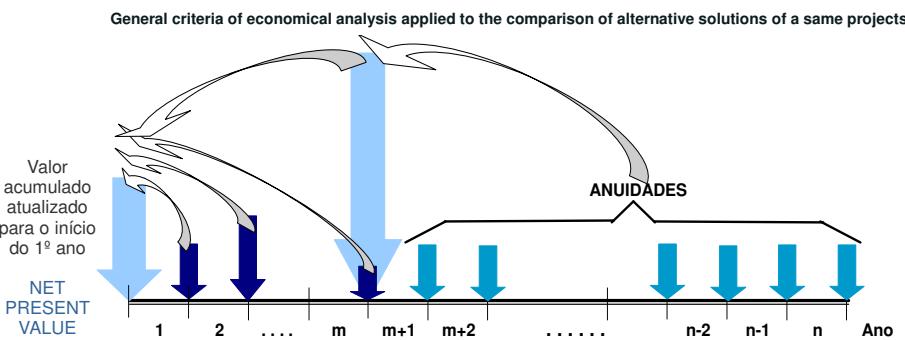
Custos de operação e de manutenção

$$S = \frac{S_i}{(1+t)^i}$$

Custo de reposição

$$R = \sum_{i=1}^{n-m} \frac{1}{(1+t)^i} R_i$$

Receitas



$$I = \sum_{i=1}^m \frac{1}{(1+t)^i} I_i$$

$$O = \sum_{i=1}^{n-m} \frac{1}{(1+t)^i} O_i$$

Investment costs

$$S = \frac{S_i}{(1+t)^i}$$

O&M annuities

$$R = \sum_{i=1}^{n-m} \frac{1}{(1+t)^i} R_i$$

Reposition costs

Incomes annuities

Each cash inflow/outflow is discounted back (...forwards) to its present (... future) value (PV). All the values thus obtained are summed = NPV (net present value)

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PT

VALOR ATUALIZADO LÍQUIDO, VAL (NET PRESENT VALUE)

O VAL representa a soma acumulada atualizada dos benefícios esperados deduzidos dos custos esperados, uns e outros durante o período de vida do projeto:

$$VAL = R - I - O - S$$

Se o VAL é negativo, o projeto deve ser rejeitado pois o valor atualizado dos benefícios não compensará o valor atualizado dos custos. Admitindo que não existem restrições à disponibilidade inicial de capital, de entre projetos alternativos com VAL positivo deve ser escolhido o que apresentar maior VAL.



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General criteria of economical analysis applied to the comparison of alternative solutions of a same projects

NET PRESENT VALUE, NPV (valor atualizado líquido, VAL)

The NPV is the difference between the present values of all the cash inflows and the present values of all the cash outflows over the period of analysis

$$NPV = R - I - O - S$$

If the NPV is negative, the project must be rejected as the net present values of the cash inflows will not compensate those of the cash outflows.

Assuming that there are no restrictions related to the initial capital availability, among alternative projects with positive NPV, the one with the highest NPV should be chosen.

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Critérios de análise económica

PT

ÍNDICE BENEFÍCIO/CUSTO, B/C (BENEFIT/COST RATIO)

O B/C representa o valor presente da “riqueza” gerada pelo projeto por unidade de “recurso atualizado utilizado”:

$$B/C = \frac{R - O}{I + S}$$

$$B/C = \frac{R}{I + S + O}$$

A primeira definição é mais coerente uma vez que agrega os fluxos monetários anuais ocorrentes durante a vida útil do projeto.

Se B/C é menor do que 1 o projeto não apresenta viabilidade. Se B/C é igual a 1 o projeto tem um interesse marginal e se é maior do que 1 o projeto é economicamente viável, sendo tanto mais viável quanto maior for B/C.

Um valor unitário de B/C conduz a um VAL igual a zero.



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BENEFIT/COST RATIO, B/C

(Índice benefício/custo)

The B/C represents the added present value of the cash inflow generated by the project per unit of the present value of the cash outflow (ratio between present values of the benefits and costs)

$$B/C = \frac{R - O}{I + S}$$

$$B/C = \frac{R}{I + S + O}$$

The first definition is more coherent as it aggregates in the numerator all the monetary flows that occur annually during the life of the project.

If B/C is less than 1 the project is not economically profitable, that is, is expected to deliver a net present value. If B/C is equal to 1 the project has a marginal interest and if it is greater than 1 the project is economically profitable, the more profitable the greater the B/C.

A unit value of B/C leads to a NPV equal to zero.

TAXA INTERNA DE RENTABILIDADE, TIR

(INTERNAL RATE OF RETURN)

A TIR é definida como sendo a taxa de atualização que torna o VAL nulo.

$$VAL = \sum_{i=1}^{n-m} \frac{1}{(1+TIR)^i} (R_i - O_i) - \sum_{i=1}^m \frac{1}{(1+TIR)^i} I_i - \frac{S_i}{(1+TIR)^i} = 0$$

É determinada de modo iterativo.

Se a taxa de atualização for igual à TIR, o VAL torna-se nulo e o B/C, unitário.

De entre projetos alternativos com diferentes TIR, o projeto mais vantajoso é o que apresentar maior TIR, sendo economicamente viável se tal taxa superar a taxa de atualização, t.

INTERNAL RATE OF RETURN, IRT

(taxa interna de rentabilidade, TIR)

- ✓ The IRT is the discount rate that makes the net present value, NPV, equal to 0 and the benefit/cost ratio, B/C, equal to 1
- ✓ Its computation is based on an iterative procedure
- ✓ Among projects/alternative solutions of a same project, the one with the highest IRT is the economically more advantageous
- ✓ If the IRT is greater than the applicable discount rate, t, the NPV will be positive and the B/C greater than 1 and the project will be economically profitable
- ✓ If the IRT is smaller than the applicable discount rate, the project/alternative solution is not economically profitable because its cash outflows are greater than the cash inflows

PERÍODO DE RECUPERAÇÃO DO INVESTIMENTO, T

(PAYBACK PERIOD)

O período de recuperação do investimento, T, é determinado com base no cash-flow acumulado atualizado e representa o número de anos até que os benefícios se compensem os custos, uns e outros acumulados atualizados. De entre projetos alternativos com diferentes T o mais vantajoso é o que apresentar menor T.

PAYBACK PERIOD

(período de recuperação do investimento)

The payback period it refers to the number of years it takes to recover the cost of an investment, in terms of present values. It is estimated based on the cumulative discounted cash flows (cashflow acumulado descontado)

Among alternative solutions of a same project, the one with the smallest payback period is the more advantageous

Years	Present Value of USD 1 @10.81%	Present value of discounted present cash flows	Discounted present value of cash flows	Cumulative present value of cash flows
0	-	-2992584	-	-
1	0.902	427258.65	385387.30	385387.30
2	0.814	427258.65	347788.50	733175.80
3	0.735	427258.65	314035.10	1047211.00
4	0.663	427258.65	283272.50	1330483.00
5	0.599	427258.65	255927.90	1586411.00
6	0.54	427258.65	230719.70	1817131.00
7	0.488	427258.65	208502.20	2025633.00
8	0.44	427258.65	187993.80	2213627.00
9	0.397	427258.65	169621.70	2383249.00
10	0.358	427258.65	152958.60	2536207.00
11	0.323	427258.65	138004.50	2674212.00
12	0.292	427258.65	124759.50	2798971.00
13	0.263	427258.65	112369.00	2911340.00
14	0.238	427258.65	101687.60	3013028.00
15	0.215	427258.65	91860.61	3104889.00



Each cash inflow and outflow is discounted back (...forwards) to its present value (PV). Then all are summed. Therefore, NPV is the sum of all terms

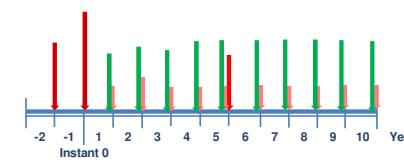


ANO	INVESTIMENTO	CUSTO ANUAL	RECEITA ANUAL	VALOR ACTUALIZADO	CASH-FLOW ACUMULADO ACTUALIZADO
- α	$I_{-\alpha}$	0	0	$VP_{-\alpha} = I_{-\alpha} (1+t)^{(\alpha-1)}$	$VP_{-\alpha}$
- β	$I_{-\beta}$	0	0	$VP_{-\beta} = I_{-\beta} (1+t)^{(\beta-1)}$	$VP_{-\alpha} + PVP_{-\beta}$
...
- γ	$I_{-\gamma}$	0	0	$VP_{-\gamma} = I_{-\gamma} (1+t)^{(\lambda-1)}$	$VP_{-\alpha} + PVP_{-\beta} + VP_{-\lambda}$
...
-1	I_{-1}	O_{-1}	R_{-1}	$VP_{-1} = I_{-1} + O_{-1} + R_{-1}$...
1	I_1	O_1	R_1	$VP_1 = (I_1 + O_1 + R_1) \frac{1}{1+t}$...
2	...	O_2	R_2
...
RECUPERAÇÃO DO INVESTIMENTO					
< 0					
> 0					
...
$n-1$	I_{n-1}	O_{n-1}	R_{n-1}	$VP_{n-1} = (I_{n-1} + O_{n-1} + R_{n-1}) \frac{1}{(1+t)^{(n-1)}}$...
n	...	O_n	R_n	$\sum_{i=-\alpha}^n VP_i$	

Cash-flow acumulado atualizado

("cumulative" discounted cash flow ... cash flow expressed in terms of "cumulative" present values)

Year, i	Investment cost, I (1000 €)	Reposition cost, S (1000 €)	Annual O&M, O&M (1000 €)	Annual incomes, R (1000 €)
-2	580			
-1	1200			
1			10	200
2			20	250
3			10	280
4			12	280
5		360	12	400
6			12	400
7			12	400
8			12	400
9			12	400
10			12	400



Discount rate, t	Cash flow			
	Discounted present value (discounted PV) (1000 €)			Cumulative discounted PV (1000 €)
0.06	I	S	O&M	R
-2	-614.80			
-1	-1200.00			-1814.80
1		-9.43	188.68	-1635.55
2		-17.80	222.50	-1430.86
3		-8.40	235.09	-1204.16
4		-9.51	221.79	-991.88
5	-269.01	-8.97	298.90	-970.95
6		-8.46	281.98	-697.43
7		-7.98	266.02	-439.39
8		-7.53	250.96	-195.95
9		-7.10	236.76	33.71
10		-6.70	223.36	250.36
Net present value, NPV (1000 €)				250.36
Benefit/cost ratio, B/C				1.1201
Payback period				year 9
Internal rate of return, IRT				0.0831

Year, i	Investment cost, I ↓ (1000 €)	Reposition cost, S ↓ (1000 €)	Annual O&M, O&M ↓ (1000 €)	Annual incomes, R ↓ (1000 €)
-2	580			
-1	1200			
1		10	200	
2		20	250	
3		10	280	
4		12	280	
5		360	12	400
6			12	400
7			12	400
8			12	400
9			12	400
10			12	400

$(1 + t)^{-i-1}$ $\frac{1}{(1 + t)^i}$ $\frac{1}{(1 + t)^{i+1}}$

Year, i	Cash flow			
	Discounted present value (discounted PV) (1000 €)			Cumulative discounted PV (1000 €)
0.06				
-2	-614.80			-614.80
-1	-1200.00			-1814.80
1		-9.43	188.68	-1635.55
2		-17.80	222.50	-1430.86
3		-8.40	235.09	-1204.16
4		-9.51	221.79	-991.88
5	-269.01	-8.97	298.90	-970.95
6		8.46	281.98	-697.43
7		-7.98	266.02	-439.39
8		-7.53	250.96	-195.95
9		-7.10	236.76	33.71
10		-6.70	223.36	250.36

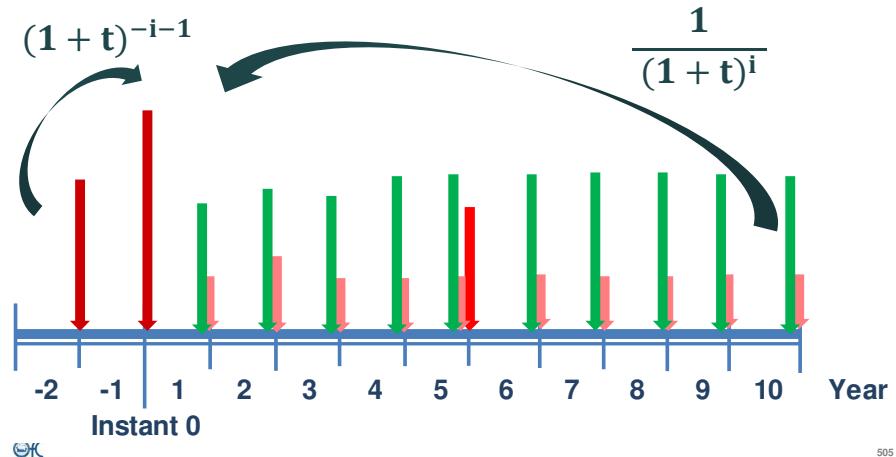
Iterative procedure imposing $NPV=0$ and changing t

Net present value, NPV (1000 €)	250.36
Benefit/cost ratio, B/C	1.1201
Payback period	year 9
Internal rate of return, IRT	0.0831

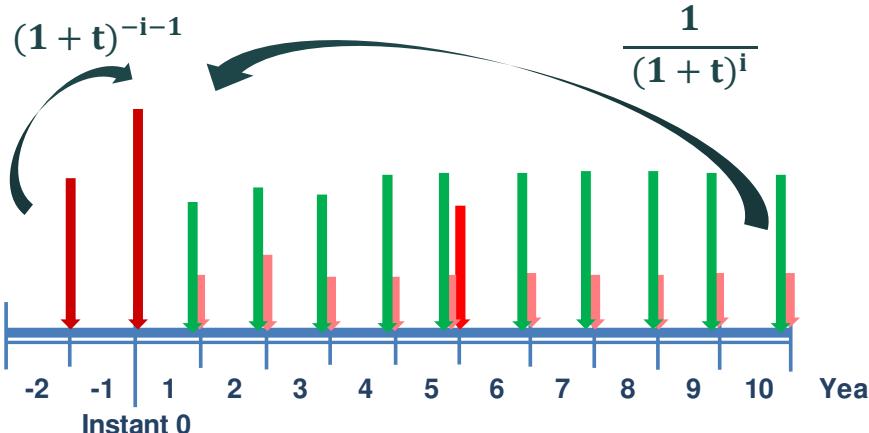
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General criteria of economical analysis applied to the comparison of alternative solutions of a same projects

It is advantageous to represent the temporal allocation of the monetary flows in order to easily identified the “gap” in time to consider when computing the different discounted present values



Regardless the notation, the exponent is always the NUMBER OF YEARS (“gap” time) between the END OF THE YEAR in which the monetary flow occurred and INSTANT 0



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- Sumários
- 1 Semestre 2020/2021
 - Enunciado do Trabalho Prático
 - Partes 1, 2 e 3 - Statement of practical work, 1st, 2nd and 3rd Parts
- Informações úteis
 - Programação das aulas e das entregas parciais do Trabalho Prático
 - Endereços de ZOOM das aulas
 - Organização do calendário da avaliação
 - Slides das aulas
 - Bibliografia 2020
 - FICHIERO PARA A AULA DE 18 DE OUTUBRO DE 2020
 - FICHIERO PARA A AULA DE 16 DE NOVEMBRO DE 2020
 - FICHIERO PARA A AULA DE 23 DE NOVEMBRO DE 2020

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(1) Período de avaliação - 07 a 19 de janeiro 2021

(2) Entrega da último trabalho - 29 de

dezembro/2020

(3) Exame - 13 de janeiro/2021 (sala 8.2.10)

(4) Orais

Janeiro de 2020 (período de avaliação de 7 a 19)

	Dia x	Dia x+1
10:00		
11:00		
12:00		
13:00		
15:00		
16:00		
17:00		
18:00		

Janeiro de 2020 (período de avaliação de 7 a 19)

	Dia x	Dia x+1
10:00		
11:00		
12:00	um dos	dias
13:00		
15:00		
16:00		
17:00		
18:00		

DEZEMBRO 2020

Seg	Ter	Qua	Qui	Sex	Sab	Dom
1	2	3	4	5	6	
7	8	9	10	11	12	13
14	15	16	17	18	19	20
21	22	23	24	25	26	27
28	29	30	31			

JANEIRO 2021

Seg	Ter	Qua	Qui	Sex	Sab	Dom
				1	2	3
4	5	6	7	8	9	10
11	12	13	14	15	16	17
18	19	20	21	22	23	24
25	26	27	28	29	30	31